# LB/DON /02 /08

THE EFFECT OF BED FRICTION ON THE PERFORMANCE
OF

FIXED BED TIDAL MODELS OF ESTUARIES

A N D

MODEL TESTING OF A STORM - WATER OVERFLOW CHANNEL

The thesis submitted to

The Department Of Civil And Structural Engineering

of

The University Of Sheffield for the degree of

Electronic Theses & Dissertations
www.lib.mrt.ac.lk
Don Charles Henry Senarath

B. Sc. Eng.

# LIBRARY UNIVERSITY OF MORATUWA, SRI LANKA MORATUWA

September 1971

PROF. D.C.H. SENARATH

University of Moratuwa

90030 <u>624 "71."</u> 624(043)

## CONTENTS

		Page
	PART ONE	
	List of symbols	1.
	Summary	3
CHAPTE	R ONE INTRODUCTION	
1-1	Tidal Models	5
1-2	Theoretical treatment of roughness	
	scale	9
1-3	Provision of roughness in tidal models	10
CHAPTE	R TWO - APPARATUS AND INSTRUMENTATION	
2-1	The tidal model	13
2-2	The tide generator University of Moratuwa, Sri Lanka.	13
2-3	Mcasurement of tidal levels	22
2-4	Measurement of velocity	. 27
2-5	Measurement of water levels in the	
	tide tank	30
CHAPTE	R THREE - FLUME TESTS	
3-1	Method of roughning the bed of the model	33
3-2	Previous investigations on the roughness	
	of surfaces	33
3 <b>-</b> 3	Determination of roughness coefficients	35
3-4	Analysis of the results of flume tests	41
CHAPTE	R FOUR - TESTING OF THE MODEL	
4-1	Preparation of the bed of the model	43
4-2	Measurement of tidal levels	48
<b>4–</b> 3	Measurement of tidal velocities	49
СНАРТЕ	R FIVE - THEORETICAL ANALYSIS AND COMPUTA	ATIONS
5-1	Flow in a tidal estuary	52
5 <b>-</b> 2	Solution of differential equations	
	representing tidal motion	54

		Page
5 <b>-</b> 3	Computation of instantaneous values of	
	width, area of cross-section, and	
	wetted perimeter	56
5-4	Numerical solution of differential	
	equations representing tidal motion	60
CHAPTE	R SIX - CONCLUSIONS	
6-1	Experimental results	64
6-2	Determination of rou{hness coefficients	65
6-3	Measurement of velocity	66
6-4	Theoretical analysis	67
6-5	Verification of the accuracy of theoretical	-
	analysis beyond section 6	68
	List of references	82
	Appendiniversity of Moratuwa, Sri Lanka.	148
	Flectronic Theses & Dissertations www.lib.mrt.ac.lk	155
	PART TWO	
	Summary	85
CHAPTE	•	
1-1	Nature of investigation	86
1-2	The prototype	86
	The model	90
_	Dissipation of energy in channels	96
CHAPTE	R TWO - TESTING OF THE MODEL	
2-1	General procedure	98
2-2	Test 1	98
2-3	Test 2	102
2-4	Test 3	102
2-5	Results of tests 2 and 3	103
2-6	Additional tests	104
2-7	Calibration of the flume	107

		Page
CHAP!	TER THREE - CONCLUSIONS	
3-1	Original design	108
3-2	Arrangement for test 2	108
3-3	Arrangement for test 3	109
3-4	Additional tests	109
3-5	Final conclusions	110
	List of references	146
	Acknowledgements	162

#### \*\*\*\*\*\*\*\*\*



## LIST OF FIGURES

## PART ONE

Fig. No.	Page
A l The hydraulic model of the Sol	way
Firth	14
A 2 The tide tank	14
A 3 The centrifugal air extractor	16
A 4 Air control valve and servo mo	tor 16
A 5 The float mechanism	17
A 6 Cam mechanism and comparing and	đ
stabilizing circuit assembly	17
A 7 Circuit diagram for comparing	and
stabilizing circuit University of Moratuwa, Sri Lan	20
A 8 Basic parts of pneumatic tide	generator 21
A 9 Peckel strain gauge apparatus	23
A 10 Servo Riter recorder	23
A 11 Balancing circuit for recording	g of depth
with the strain gauge apparatu	s 24
A 12 Float gauge and electrical pro	bes
connected to the balancing box	26
A 13 Miniature current meter manufac	ctured
by Gloster Equipment Ltd.	26
A 14 Miniature current meter manufac	ctured
by Kent Industrial Instruments	Ltd. 29
A 15 Measuring head used with the cu	urrent
meters	29
A 16 Principle of determination of	water
level in the tide tank	32
A 17 Variation of Manning's coeffic:	ient
with depth of flow	39
A 18 Variation of Manning's coeffic:	ient
with the ratio of depth of flow	w to
diameter of cement blocks	40

Fig. No.	Page
A 19 Plan of the hydraulic model of the	
Solway Firth	44
A 20 Arrangement of hemispherical cement	
blocks on the bed of the model	47
A 21 Longitudinal section of the model sho	wing
the meaning of extrapolated level at	low
tide	58
A 22 Observed tide curve at section 1	70
A 23 Tidal velocities recorded at section	1
in test 1 A	71
A 24 Tidal velocities recorded at section	1
in test 1 B	72
A 25 Tidal velocities recorded at section	1
in test 1 C	73
A 26 Variation of water level in the tide	
Electronic Theses & Dissertations	74
A 27 Tidal velocities at section 1 compute	d.
from the observed variation of water	
level in the tide tank	<b>7</b> 5
A 28 Observed tide curves at section 6	<b>7</b> 6
A 29 Tide curves at section 6 computed fro	m
tidal elevations and velocities obser	ved
at section 1	77
A 30 Tidal velocities recorded at section	6 78
A 31 Observed tide curves at section 13	79
A 32 Tidal velocities recorded at section 1	.3 80
A 33 Observed tide curves at section 19	81
A 34 Variation of $n\sqrt{S/d}$ as a function of y	/d 156
A 35 Variation of $log_{10}(n\sqrt{S/d}-0.03)$ as a	
function of y/d	1.5 <b>7</b>
A 36 Observed tide curve at section 6 in	
test 2	158
A 37 Observed mean tidal velocity curve	
at section 6 in test 2	159
A 38 Tide curves at section 13 in test 2	160
A 39 Tide curves at section 19 in test 2	161

### PART TWO

Fig.No.		Page
ві	Arrangement of cascades in storm-water	
	overflow channel	89
B 2	Shapes of appurtenances	91
В 3	Original arrangement of appurtenances	
	D.S.of cascade 1 A	92
B 4	Hydraulic model of the storm-water	
	overflow channel	95
B 5	Results of Bradley and Peterka for	
	stilling basin 1 - Variation of %loss	
	of energy with inflow Froude number	111
В 6	Results of Bradley and Peterka for	
	stilling basin 1 - Variation of the	
	rationore(sign gth of a fining in that adepth)	
	with Elagrowif Touses numbiscertations	112
B 7	Water surface profiles in test 1	
	D.S.of cascades 1A and 1B	113
B 8	Water surface profiles in test 1	
	D.S.of cascade 2	114
B 9	Flow D.S. of cascade 1 A Test 1 at	
	peak discharge	115
B 10	Flow D.S. of cascade 1 A test 1 at	
	75% peak discharge	115
B 11	Flow D.S.of cascade 1 A test 1 at	
	50% peak discharge	116
B 12	Flow D.S.of cascade 1 A test 1 at	
	25% peak discharge	116
B 13	Flow D.S. of cascade 1 B test 1 at	
	peak discharge	117
B 14	Flow D.S. of cascade 1 B test 1 at	
	75% peak discharge	117
B 15	Flow D.S.of cascade 1 B test 1 at	
	50% peak discharge	118
B 16	Flow D.S. of cascade 1 B test 1 at	
	25% neak discharge	118

Fig. No.	Page
B 17 Flow D.S.of cascade 2 test 1 at	
peak discharge	119
B 18 Flow D.S. of cascade 2 test 1 at	
75% peak discharge	119
B 19 Flow D.S. of cascade 2 test 1 at	
50% peak discharge	120
B 20 Flow D.S. of cascade 2 test 1 at	
25% peak discharge	120
B 21 Variation of Froude number with the	
mean depth of flow in the channel	121
B 22 Variation of the mean velocity of flow	
with the mean depth of flow in the channel	125
B 23 Water surface profiles in test 2	
D.S.of cascades 1 A and 1 B	126
B 24 Water surface profiles in test 2	•
S Precessed theses & Dissertations	127
B 25 Flow P.S. of reasonde 1 B test 2 at	
peak discharge	128
B 26 Flow D.S. of cascade 1 B test 2 at	
75% peak discharge	128
B 27 Flow D.S. of cascade 1 B test 2 at	
50% peak discharge	129
B 28 Flow D.S. of cascade 1 B test 2 at	
25% peak discharge	129
B 29 Water surface profiles in test 3	
D.S.of cascades 1 A and 1 B	130
B 30 Water surface profiles in test 3	
D.S.of cascade 2	131
B 31 Flow D.S.of cascade 1 A test 3 at	- 20
peak discharge	132
B 32 Flow D.S.of cascade 1 A test 3 at	- 30
75% peak discharge	132
B 33 Flow D.S.of cascade 1 A test 3 at	7.00
50% peak discharge	133
B 34 Flow D.S. of cascade 1 A test 3 at	7 7 7
25% peak discharge	133

Fig. No.	Page
B 35 Flow D.S.of cascade 1 B test 3 at	
peak discharge	134
B 36 Flow D.S. of cascade 1 B test 3 at	
75% peak discharge	134
B 37 Flow D.S. of cascade 1 B test 3 at	
50% peak discharge	135
B 38 Flow D.S. of cascade 1 B test 3 at	
25% peak discharge	135
B 39 Flow D.S.of cascade 2 test 3 at	
peak discharge	136
B 40 Flow D.S. of cascade 2 test 3 at	
75% peak discharge	136
B 41 Flow D.S.of cascade 2 test 3 at	
50% peak discharge	137
B 42 Flow Disonftycasdaden Zwtestri Latka.	
25% peakronischarges & Dissertations	137
B 43 Flowwinwtles.tn4t.ac.lk	139
B 44 Flow in test 5	139
B 45 Flow in test 6	140
B 46 Flow in test 7	140
B 47 Flow in test 8	141
B 48 Flow in test 9	141
B 49 Flow in test 10	142
B 50 Flow in test 11	142
B 51 Flow in test 12	143
B 52 Calibration of the Venturi-flume	145

\*\*\*\*\*

### LIST OF TABLES

Table	Po.	age
1	Variation of the mean depth of flow with distance along the channel D.S. of cascade 1 A in the absence of any appurtenances	122
2	Variation of the mean depth of flow with distance along the channel D.S. of cascade 1 B in the absence of any	
	appurtenances	123
3	Variation of the mean depth of flow with distance along the channel D.S. of leasestee 2 Marthevabsche any	
4	apporteninglesses & Dissertations www.lib.mrt.ac.lk Mean velocities of flow in the channel at different rates of flow in the	124
	different tests	138
5	Calibration of the Venturi-flume	144

\*\*\*\*\*

#### PART ONE

THE EFFECT OF BED FRICTION ON THE PERFORMANCE OF FIXED BED TIDAL MODELS OF ESTUARIES



List Of Symbols.

The following symbols are used except where otherwise mentioned.

Suffixes m and p refer to model and prototype respectively.

A = Area of cross-section

b = Width of Section

C = Chezy coefficient

c = Wave celerity

d = Diameter of hemispherical cement blocks.

e = Vertical exaggeration =  $\frac{Vs}{Hs}$ ; Also diameter of uniform hemispherical sand grain. University of Moratuwa, Sri Lanka.

f = Africtionreacfficient Dissertations

g = Acceleration duet to gravity.

H = Elevation of water level measured with respect to the
mean water level at section l .

 $Hs = Horizontal Scale = \frac{Xm}{Xp}$ 

h = Depth of water in the model.

i = Bed Slope

L = Wave length

n = Manning Coefficient

P = Wetted perimeter

R = Hydraulic mean depth

ro = pipe radius measured to crests of roughness elements.

S = Centre to Centre spacing of hemispherical cement blocks.

T = Tidal period

t = Time

U = Horizontal velocity

 $Vs= \ Vertical \ scale = \frac{Ym}{Yp}$ 

X = Horizontal distance along the estuary

Y = Vertical distance

y = Depth of water in the flume measured up to the flat bed of the flume.



#### SUMMARY

This report describes an investigation that was conducted on a fixed bed hydraulic model of the Solway Firth. The aim of the investigation was to study the manner in which the roughness of the bed of model influences the performance of the model regarding tidal levels and tidal velocities.

Models of estuaries are usually constructed to a distorted scale and it is not possible to derive theoretically, the correct magnitude of bed roughness necessary in order to obtain dynamic similarity. In this investigation, it was intended to study the extent to which Electronic Theses & Dissertations the tidal velocities mand elevations depend on bed friction and also explore the possibility of ascertaining what magnitude of bed roughness of model would produce dynamic similarity between a given model and prototype.

The method adopted in this investigation was to produce roughnesses of known magnitude on the bed of the model and study the performance of the model under these roughnesses. Bed roughness was produced by fixing hemispherical cement blocks on the bed of the model according to predetermined patterns. By means of a series of tests in a rectangular flume under uniform flow conditions, the patterns required to produce the desired magnitudes of roughness were determined. Tidal velocities and Elevations were observed at seven points of the model for three different roughnesses of the bed.

A theoretical analysis of the flow in the model was also carried out taking into account, the bed roughness. As a result of this analysis it was found possible to represent the flow in the model by means of a mathematical equation adapted to a computer program. The results of this mathematical analysis showed agreement with the observations made. This agreement failed in certain parts of the model which remained dry during a certain period of the tidal cycle causing discontinuity of the water level as a function of time.

Although it is not possible to calculate the correct magnitude of bed roughness for the model, it has been found possible with the book of theoretical analysis of flow in the model, to select a suitable magnitude of bed roughness which produces the desired behaviour of the model as regards tidal levels. However, it has not been possible in this investigation to verify the accuracy of the representation of tidal velocities in the mathematical treatment owing to the difficulty of measuring the mean tidal velocities, since in most regions of the model, the depths of flow were insufficient for accurate velocity measurement.

In the range of bed roughnesses employed in this investigation, it has been found that the effect of bed roughness on the tidal elevations is comparatively less significant than its effect on tidal velocities.