# FEASIBILITY OF USING PRESENTLY USED PRE-CAST SECTIONS FOR M-BEAM TYPE

**BRIDGES** 



A thesis submitted for the partial fulfillment of the Degree of Master of Engineering in Structural Engineering Design



Submitted by Shyama Liyanage February 2003

University of Moratuwa

77704

Supervised by

Dr. M. T. R. Jayasinghe

**Associate Professor** 

**Department of Civil Engineering** 

University of Moratuwa

624 "0" 624.21(8.18.7)

77704

#### Abstract

For the rapid economic development of Sri Lanka, the upgrading of transportation facilities is one of the main criteria to be fulfilled. In this context, there may be an increase in the construction of bridges which will need more money. This can not be afforded by a developing country like Sri Lanka, which can not allocate too much money in construction work. So it is advantageous to carryout research, aimed at reducing the cost of them.

Most commonly used deck type for the bridges constructed in Sri Lanka is pseudo slab type bridge decks. For this type of decks, inverted T-beams are used with in-filled decks. So the cost of these types of bridge decks can be considerable. This can be reduced to some extent by adopting the M-beam type of construction with in-situ top slab. For this type of construction, it was decided to check with inverted T-beam sections, since they are readily available in Sri Lanka.

University of Moratuwa, Sri Lanka,

Feasible spans of the standard sections of inverted T-beams for top slab constructions were found by trial and error. Magnel diagrams were drawn to get the feasible regions of cables positions. To facilitate the checking of these sections a spreadsheet was prepared to do the design calculations. A cost comparison was done to find the most economical deck type out of two types of bridge decks. From the results got by the case study, it can be seen that the available T-beam sections can be used for the shorter spans with the top slab construction than when they are used in the in-filled construction. Even though the reduction of span is about 2.0 m to 2.5 m, the reduction of dead weight of the super structure is considerable which may cause the reduction of foundation sizes, especially in the case of shallow foundations. As per the results got from the cost study, it was found that the reduction of cost of construction of the superstructure is also reduced to some extent due to the adoption of the top slab construction.

**Acknowledgements** 

My sincere thanks to the project supervisor, Dr. M. T. R. Jayasinghe for devoting his

valuable time in guiding me to complete the research study. It is no doubt that without his

interest and guidance this would not have been a success.

I wish to thank the Vice Chancellor, Dean of the Faculty of Engineering and the Head,

Department of Civil Engineering for allowing me to use the facilities available at the

University of Moratuwa.

I am grateful to the Road Construction & Development Company for the financial

assistance given to me to follow this postgraduate degree course. I am also grateful to the

Road Development Authority for the allowance given me to get the relevant data for this

thesis.

I wish to thank Dr. (Mrs.) M. T. P. Hettiarachchi, the course and research coordinator of

the project for the encouragement given to me in completing this study, and all the

lecturers of the postgraduate course on Structural Engineering Design who helped to

enhance my knowledge.

I appreciate very much the help given by Dr. (Mrs.) Chintha Jayasinghe to do the design

calculations.

I would like to dedicate this hard work to my parents and my son for their enormous

support. Finally, I wish to thank everybody who helped me in numerous ways in

completing my research study.

Shyama Liyanage

Road Construction and Development Company (Pvt.) Ltd.

ii

### Contents

Abstract			
Acknowledg	gements	•	i
Contents			ii
List of Figu	res		v
List of Table	es		vii
Chapter -	1.0	Introduction	1
	1.1	General	•
	1.2	Types of bridges constructed in Sri Lanka	2
	1.3	Types of pre-cast pre-tensioned bridge decks	3
	1.4	The objectives of the study	4
	1.5	The methodology	4
	1.6	The main findings of the project	4
	1.7	The arrangement of the thesis University of Moratuwa, Sri Lanka, Electronic Theses & Dissertations www.lib.mrt.ac.lk	4
Chapter -	2.0	Literature review	(
	2.1	Historical development	6
	2.2	Comparison with other materials used for structural construction	8
	2.3	Forms of pre-stressing	ç
	2.4	Use of pre-stressed concrete members in bridges	11
	2.5	Summary	22
Chapter -	3.0	Computer based design of pre-stressed concrete bridge decks	23
	3.1	General	23
	3.2	The sections used in Sri Lanka	23
	3.3	Selection of preliminary dimensions and method of analysis	26
	3.4	Selection of the grillage model	26
	3.5	Determination of Magnel inequalities	31

	3.0	Design of pre-stressed beam for top stab construction	33
	3.7	Summary	42
Chapter -	4.0	Main findings of the project	<b>4</b> 4
	4.1	General	44
	4.2	Grillage outputs for the CASE STUDY	44
	4.3	Grillage outputs for in-filled deck (torsional system)	52
	4.4	Feasible spans of standard sections with top slab construction	54
	4.5	Comparison of weight of two types of bridge decks	54
	4.7	Comparison of cost of construction	55
	4.8	Summary	58
Chapter -	5.0	Conclusions and future works	59
	5.1	Conclusions	59
	5.2	Future works University of Moratuwa, Sri Lanka. Electronic Theses & Dissertations www.lib.mrt.ac.lk	60
References			61
Appendix – A		Design calculations for an inverted T-beam section with top slab construction	63
	A.1	Design data	63
	A.2	Notation	65
	A.3	Sign convention	66
	A.4	Magnel inequalities	66
	A.5	Determination of pre-stress and eccentricity	68
	A.6	Calculation of transmission length	69
	A.7	Calculation of effective span	69
	A.8	Checking of stresses at transfer stage	69
	A.9	Checking of stresses at service stage	73
·	A.10	Design for ultimate limit state	84
	Δ 11	Check for deflection	98

	•	
Appendix – B	Spreadsheet developed for the CASE STUDY	102
B.1	Spreadsheet for 7.5 m span	103
B.2	Spreadsheet for 9.5 m span	110
B.3	Spreadsheet for 11.0 m span	117
B.4	Spreadsheet for 12.0 m span	124
B.5	Spreadsheet for 13.0 m span	131
B.6	Spreadsheet for 14.0 m span	138
Appendix – C	Calculations for the comparison of weights of two types of bridge decks	145
C.1	Assumptions	145
C.2	9.5 m span bridge deck	145
. C.3	11.0 m span bridge deck	147
C.4	12.0 m span bridge deck	148
C.5	13.0 m span bridge deck	149
C.6	14.0 m span bridge deck	150
Appendix – D	Calculations for comparison of cost of construction for two types of bridge decks	152
D.1	Assumptions lectronic Theses & Dissertations	152
D 2	Calculation of cost and comparison of them	152

## List of Figures

Figure 1.1	Pseudo slab	-
Figure 1.2	Top slab	
Figure 1.3	Top and bottom slab	-
Figure 2.1	Barrel staves compressed with hoops	(
Figure 2.2	Spokes of bicycle wheel	6
Figure 2.3	A composite section with castellation to provide composite action	14
Figure 2.4	Composite sections with stirrups to provide composite action	14
Figure 2.5	Composite deck with box beams	17
Figure 2.6	Standard sections of M-beam	19
Figure 2.7	Standard sections of T-beam	20
Figure 3.1	Standard inverted T-beam sections, available in Sri Lanka	24
Figure 3.2	Strand arrangements of standard beam sections	25
Figure 3.3	A grillage model used for the analysis	27
Figure 3.4	Actual section	28
Figure 3.5	Idealized section  University of Moratuwa, Sri Lanka.  Electronic Theses & Dissertations  www.lib.mrt.ac.lk	28
Figure 3.6	Transverse member	29
Figure 3.7	End diaphragm	29
Figure 3.8	Sign convention	31
Figure 3.9	Magnel diagram	33
Figure 3.10	Forces and moments due to differential shrinkage	36
Figure 3.11	Stresses due to differential shrinkage	36
Figure 3.12	Dimensions for longitudinal shear	41
Figure 4.1	Bending moment diagrams of a bridge deck with top slab construction for combination of HA and HB loads (13.5 m span)	51
Figure 4.2	Shear force diagrams of a bridge deck with top slab construction for combination of HA and HB loads (13.5 m span)	51
Figure 4.3	Bending moment diagrams of a bridge deck with in-filled construction for combination of HA and HB loads (13.5 m span)	52
Figure 4.4	Torsional moment diagrams of a bridge deck with in-filled construction for combination of HA and HB loads (13.5 m span)	53
Figure 4.5	Shear force diagrams of a bridge deck with in-filled construction for combination of HA and HB loads (13.5 m span)	53
Figure A.1	Actual section	63

Figure A.2	Idealized section	63
Figure A.3	Sign convention	66
Figure A.4	Magnel diagram	67
Figure A.5	Arrangement of wires	68
Figure A.6	Effective span	69
Figure A.7	Stress diagram due to pre-stress and self weight of the beam	69
Figure A.8	Bending moment at a distance; x	70
Figure A.9	Stress diagram due to differential shrinkage (after 4 months)	77
Figure A.10	Stress diagram due to differential shrinkage (after 3 weeks)	78
Figure A.11	Stress diagram due to differential creep (after 4 months)	79
Figure A.12	Stress diagram due to differential creep (after 3 weeks)	80
Figure A.13	Positive temperature differences	81
Figure A.14	Stresses due to positive temperature differences	82
Figure A.15	Negative temperature differences	82
Figure A.16	Stresses due to negative temperature differences	83
Figure A.17	Values got from the Figure-3 of the code	84
Figure A.18	Stress and strain diagrams of Moraluwa, Sri Lanka.	85
Figure A.19	Variation of pre-stress along the beam	87
Figure A.20	Dimensions for longitudinal shear	96
Figure A.21	Bending moment diagram due to an UDL	99
Figure A.22	Bending moment diagram due to pre-stress	99
Figure A.23	Bending moment diagram due to a KEL	100
Figure C.1	Cross section of a top slab deck	145
Figure C.2	Cross section of an in-filled deck	145
Figure C.3	Beam section used for the span of 9.5 m in-filled deck	146
Figure C.4	Beam section used for the span of 9.5 m top slab deck	146
Figure C.5	Beam section used for the span of 11.5 m in-filled deck	147
Figure C.6	Beam section used for the span of 11.0 m top slab deck	147
Figure C.7	Beam section used for the span of 11.5 m in-filled deck	148
Figure C.8.	Beam section used for the span of 12.0 m top slab deck	148
Figure C.9	Beam section used for the span of 13.5 m in-filled deck	149
Figure C.10	Beam section used for the span of 13.0 m top slab deck	149
Figure C.11	Beam section used for the span of 14.5 m in-filled deck	151
Figure C 12	Ream section used for the span of 14.0 m top slab deck	151

#### List of Tables

Table 1.1	Types of bridges constructed in Sri Lanka	2
Table 4.1	Results for the 7.5 m span	45
Table 4.2	Results for the 9.5 m span	46
Table 4.3	Results for the 11.0 m span	47
Table 4.4	Results for the 12.0 m span	48
Table 4.5	Results for the 13.0 m span	49
Table 4.6	Results for the 14.0 m span	50
Table 4.7	Comparison of torsional and torsion less systems	52
Table 4.8	Feasible spans for top slab construction	54
Table 4.9	Comparison of weights	55
Table 4.10	Size of void forms for in-filled construction	56
Table 4.11	Cost comparison	57
Table A.1	Sectional properties	63

